

SUBJECT

# Building Technology II

## Assignment B- Foundation Design

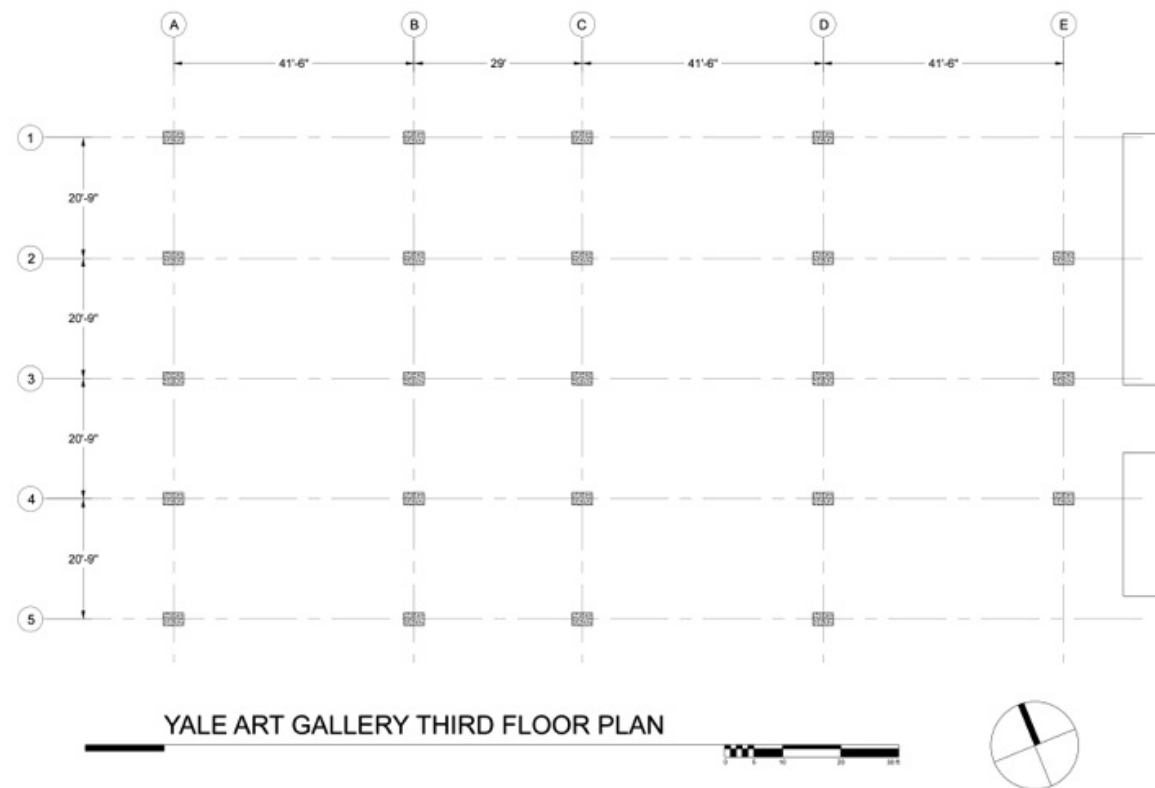
DATE

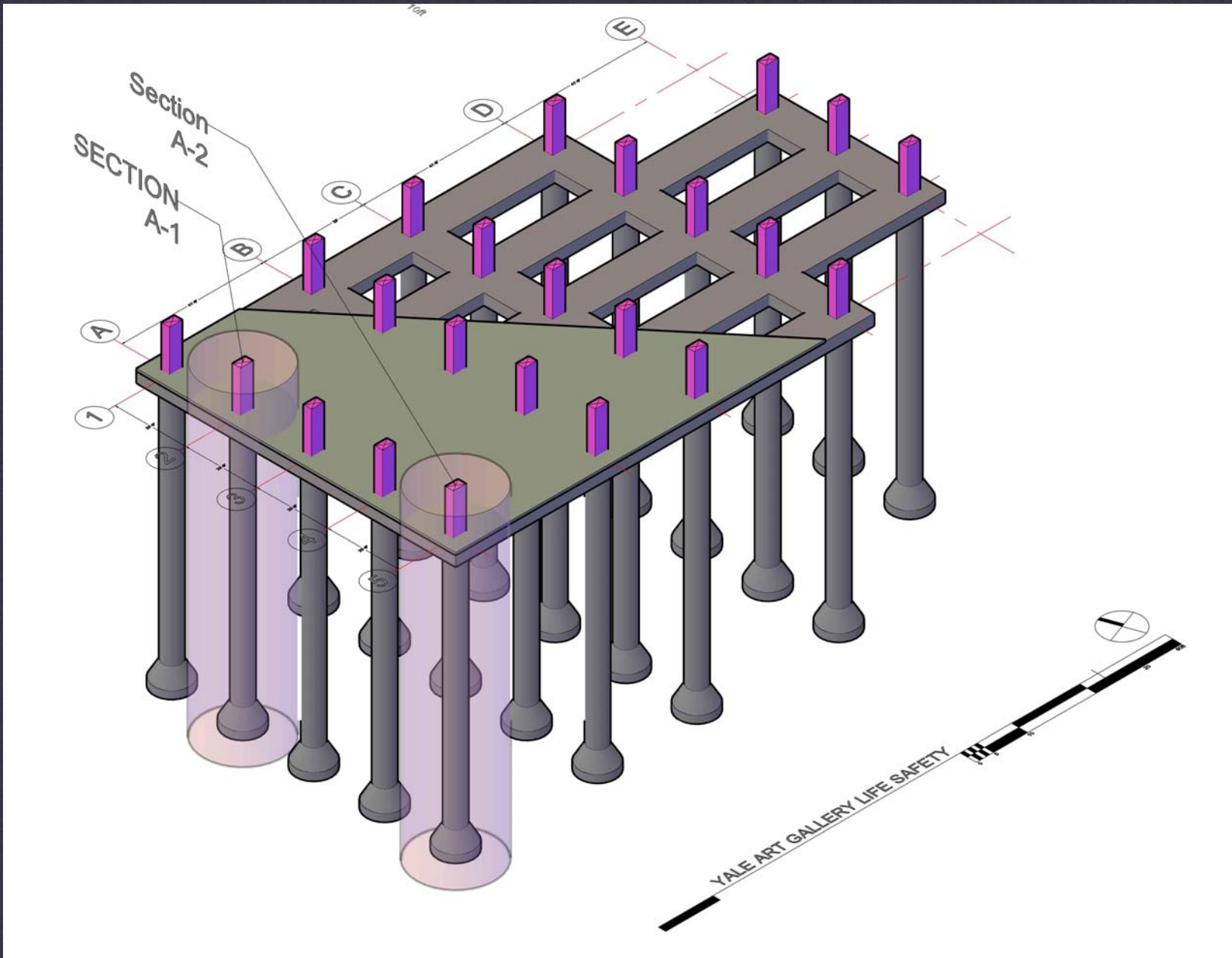
FALL 2012

PROFESSOR

Friedman

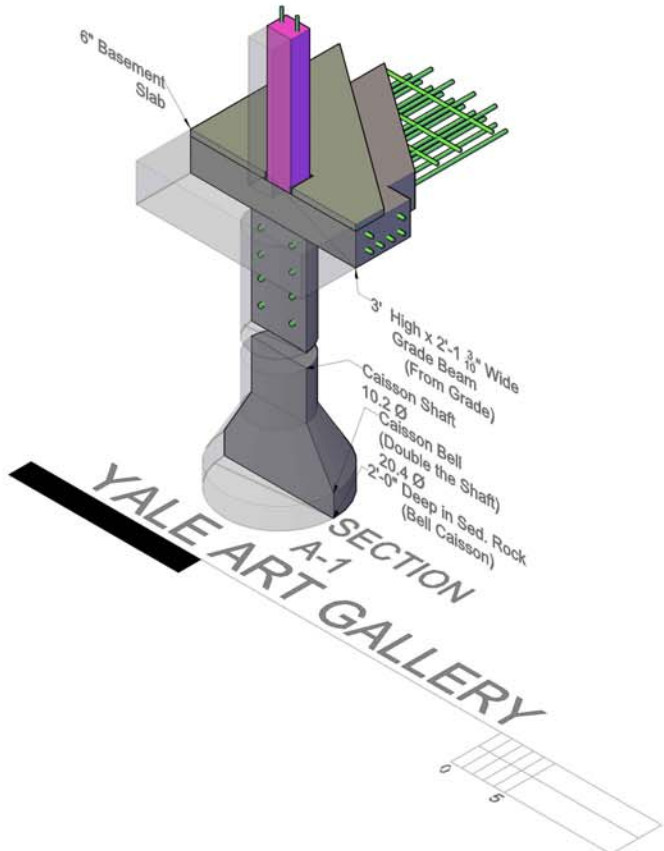
We are going to build off of Assignment A and take our 2D columns and extend this information into 3D space.





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**A**  
TOTAL LOAD: 7.5 MILLION POUNDS

**B**  
STRUCTURE:  
Area = Length x Width  
23 COLUMNS @ 3'-6" X 2'-2" = 7.58 SQFT/ COLUMN  
23 COLUMNS X 7.58 SQFT/COLUMN=174.34SQFT  
(GOING INTO FOUNDATION)

**A/B:**  
Total Loads / Structural Area  
7.5 MILLION POUNDS / 174.34 SQFT  
= 43,019 LBS/SQFT

**C:**  
SEDIMENTARY ROCK ALLOWABLE  
FOUNDATION PRESSURE = 4,000 PSFSQ  
Total Loads / Allowable Foundation Pressure  
7.5 MIL LBS / 4000 PSFSQ = 81.52 SQFT  
Total Area per Footing  
81.52  
Square root of Total Area Per footing is 9.28  
THE FOOTING SHOULD BE 9'-1" X 9'-1" if "SQUARE"

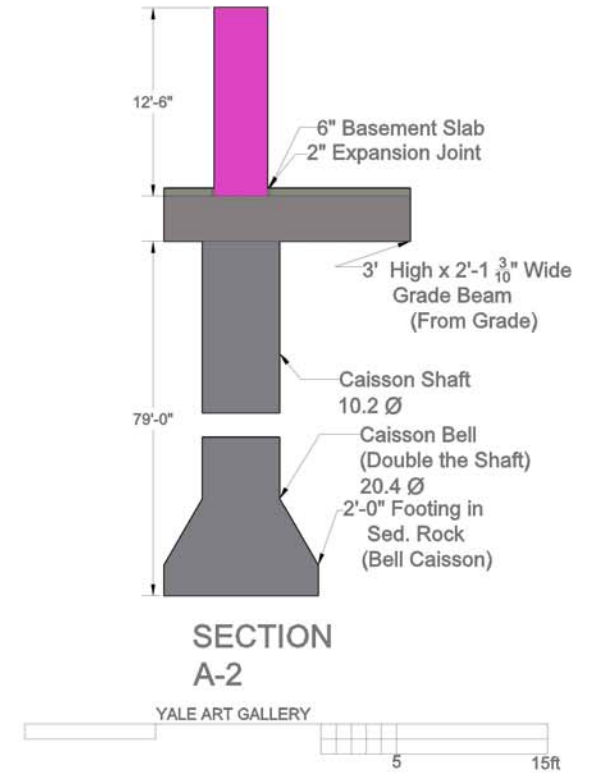
Used Footing CAISSON

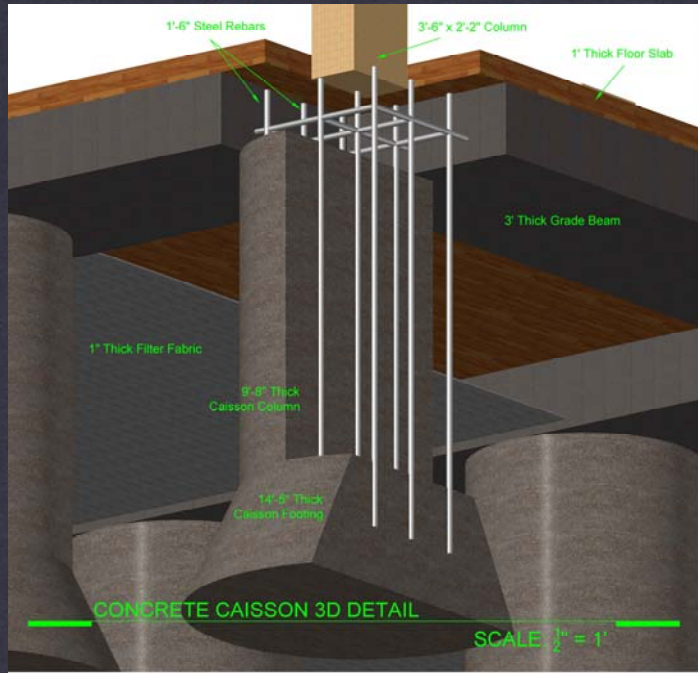
$$\text{Area} = 3.14 (r)^2$$

$$r^2 = \text{Area} / 3.14$$

$$81.52 = 3.14 (R) \text{ SQ}$$

SO, RADIUS OF CAISSON = 5.09'





**Footing Dimension Analysis**

A) Weight of Building = 7,500,000 lbs  
 B) Soil Strength Capacity = 2,000 psf  
 C) Column Size: 1'-6" x 2'-2" x 3'-5", 2,167'  
 D) Number of columns: 23  
 E) Total area of structural columns:  $(23 \times 21.67) \times (23) = 174.44 \text{ sf}$

Step 1:  
 $\frac{7,500,000}{2,000} = 3,750 \text{ sq ft} > 174.44 \text{ sq ft}$  of soil bearing capacity so footings are needed

Step 2:  
 $\frac{7,500,000}{2,000} = 3,750 \text{ sq ft}$

Step 3:  
 $\frac{3,750}{23} = 163 \text{ sq ft per footing}$

Step 4:  
**Shallow Foundation**  
 Dimensions of Footing =  $\sqrt{163} = 12.57' \times 12.57' \times 1.0'$   
**Deep Foundation**  
 Dimensions of Caisson: 163 =  $\pi r^2$  then radius = 7.2 ft  
 so diameter = 14.4 ft = 14' 0"

**Foundation Price Analysis**

**Deep Foundation**

Step 1:  
 Volume of cylindrical caisson = (volume of caisson column) + (volume of caisson footing)  
 Volume of caisson column =  $\pi r^2 h = \pi (7.2)^2 (12.57) = 948 \text{ cf}$   
 Volume of caisson footing =  $\pi r^2 h = \pi (7.2)^2 (2.0) = 328 \text{ cf}$   
 Volume of cylindrical caisson =  $948 \text{ cf} + 328 \text{ cf} = 1,276 \text{ cf per caisson}$

Step 2:  
 Price of concrete for caisson = (volume of cylindrical caisson) x (price of concrete per cf) x (number of caissons)  
 Price of concrete for caisson =  $(1,276 \text{ cf}) \times (131.11 \text{ per cf}) \times (23) = 3,656,000$

Price of drilling = (price of drilling one caisson hole) x (number of caissons)  
 Price of drilling =  $(561.25) \times (23) = 12,809,000$

Total price of deep foundation =  $(3,656,000) + (12,809,000) = 16,465,000$

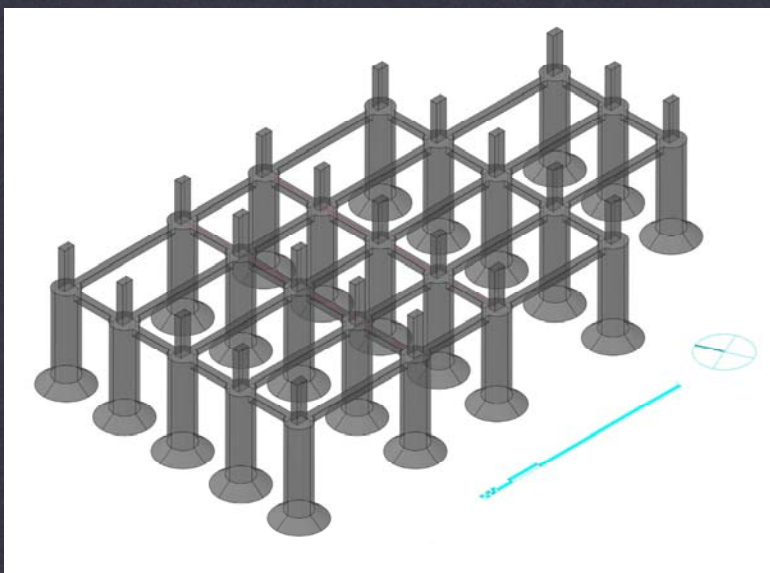
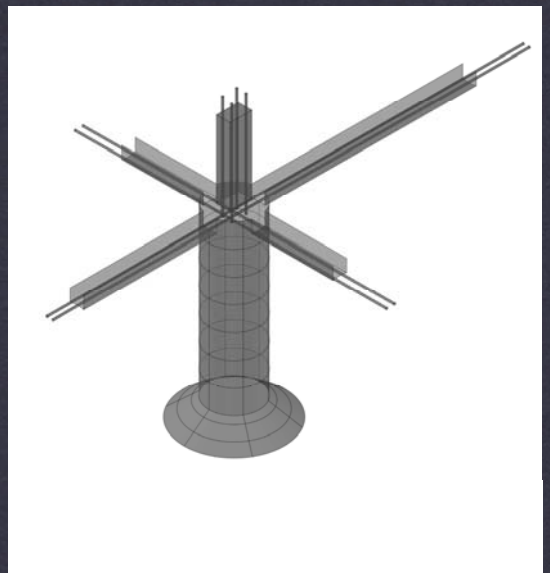
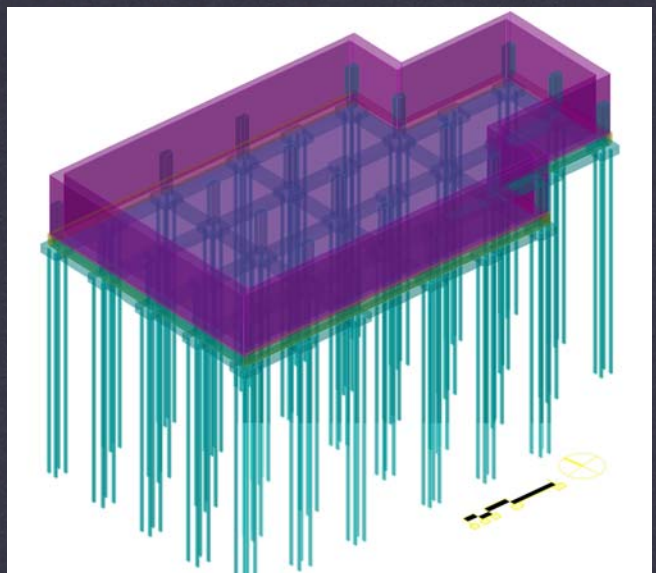
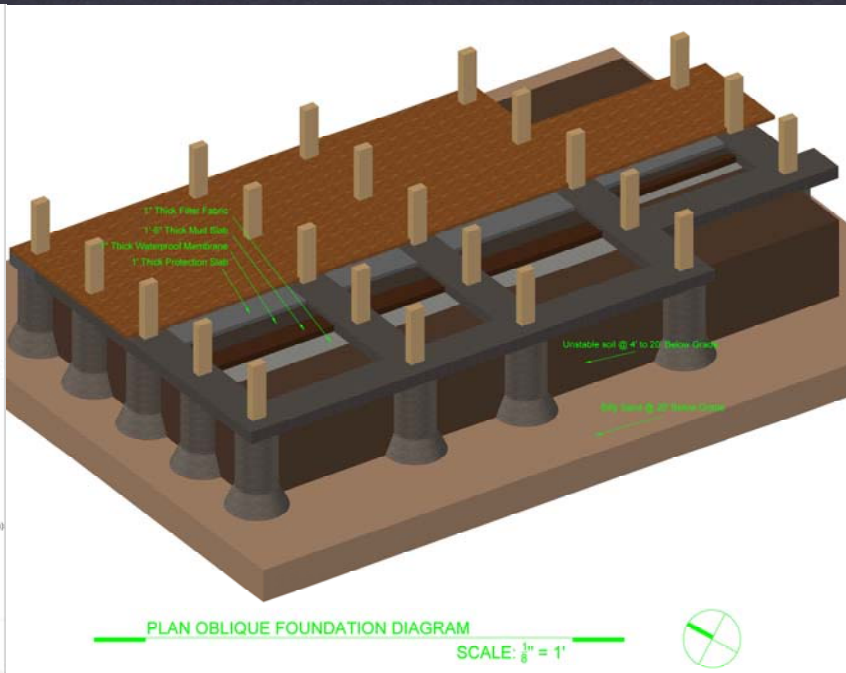
**Shallow Foundation**

Step 1:  
 Volume of column footing = (volume of column space) + (volume of column footing)  
 Volume of column space = (volume of caisson column) = 948 cf  
 Volume of column footing = (area per footing) x (footing depth) =  $(163 \text{ sf}) \times 1' = 163 \text{ cf}$   
 Volume of column footing =  $948 \text{ cf} + 163 \text{ cf} = 1,111 \text{ cf per column footing}$

Step 2:  
 Price of concrete for column footing = (volume of column footing) x (price of concrete per cf) x (number of caissons)  
 Price of concrete for column footing =  $(1,111) \times (131.11) \times (23) = 3,261,000$

Price of excavation = (gross area of basement floor) x (depth) x (price per cf of excavated soil)  
 Price of excavation =  $(12,800 \text{ ft}^2 \times 20 \text{ ft}) \times (84) = 215,040,000$

Total price of shallow foundation =  $(3,261,000) + (215,040,000) = 218,301,000$



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# Assignment B

## CASE STUDY #1: Foundation Design

**DUE: SEPT 21, 2012**

• 2 boards required:

- i. Foundation Plan @  $3/32" = 1'-0"$
- ii. Foundation Section @  $1/2" = 1'-0"$
- iii. 3D Foundation Model @  $3/32" = 1'-0"$
- iv. 3D Section Model @  $1/2" = 1'-0"$
- v. Foundation Calculations

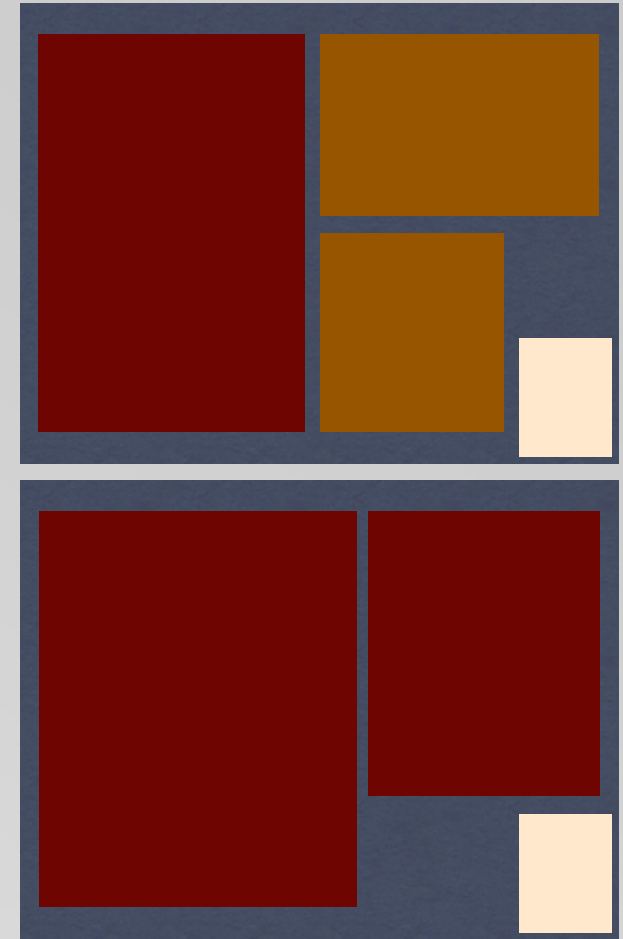
• All views must have a north arrow

• One graphic scale must be included for each unique scale

• All sheets must use the titleblock from Assignment A (change the label to Assignment B and the Dwg number to A-200 and A-201)

• Site maps to include the following:

- i. Scale
- ii. Altitude
- iii. Latitude and Longitude Coordinates of Case Study Building
- iv. Case Study Building Outlined/Highlighted



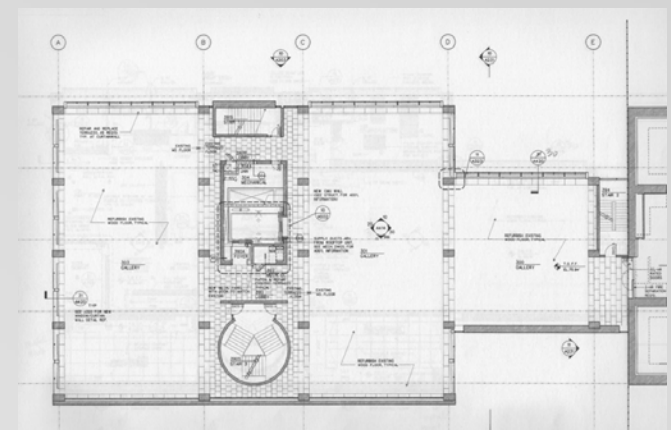
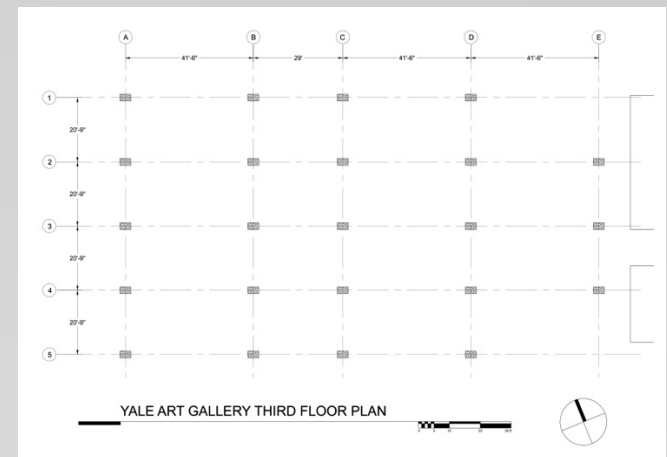
# assignment B

## CASE STUDY #1: Foundation Design

**DUE: SEPT 21, 2012**

- ✿ 24" x 36" title block that follows studio standards
- ✿ emphasis on quality of draftsmanship including:
  - ✿ layer management
  - ✿ line weights
  - ✿ grid dimensions
  - ✿ standard notations (doors, elevators, stairs\_refer to Arch Graphic Standards)
- ✿ column size: 26" x 42"
- ✿ submit PDFs and Zip files with all x-refs and rasters included.

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# assignment B

## CASE STUDY #1: Foundation Design

**DUE: SEPT 21, 2012**

•Class will be broken up into 4 groups:

•**Group 1:**

•Sandy Clay: 0-30'

•Clay: 30-60'

•**Group 2:**

•**\***Organic Peat: 0- 40'

•**\***Sand: 40-60'

•**Group 3:**

Sandy Gravel: 0-40'

Silty Clay: 40-60'

•**Group 4:**

Organic Peat: 0- 60'

Sedimentary Rock: 60'+



**TABLE 1804.2  
ALLOWABLE FOUNDATION AND LATERAL PRESSURE**

CLASS OF MATERIALS	ALLOWABLE FOUNDATION PRESSURE (psf) <sup>d</sup>	LATERAL BEARING (psf/ft below natural grade) <sup>d</sup>	LATERAL SLIDING	
			Coefficient of friction <sup>a</sup>	Resistance (psf) <sup>b</sup>
1. Crystalline bedrock	12,000	1,200	0.70	—
2. Sedimentary and foliated rock	4,000	400	0.35	—
3. Sandy gravel and/or gravel (GW and GP)	3,000	200	0.35	—
4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SP, SM, SC, GM and GC)	2,000	150	0.25	—
5. Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)	1,500 <sup>c</sup>	100	—	130

For SI: 1 pound per square foot = 0.0479 kPa, 1 pound per square foot per foot = 0.157 kPa/m.

- Coefficient to be multiplied by the dead load.
- Lateral sliding resistance value to be multiplied by the contact area, as limited by Section 1804.3.
- Where the building official determines that in-place soils with an allowable bearing capacity of less than 1,500 psf are likely to be present at the site, the allowable bearing capacity shall be determined by a soils investigation.
- An increase of one-third is permitted when considering load combinations, including wind or earthquake loads, as permitted by Section 1605.3.2.

## FOUNDATION CALCULATIONS

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-Building Height = 5 floors

-Total weight /load of the building (dead + live loads) = 7,500,000 lbs.

-Column size = 3'-6" x 2'-2"

-Frost line of New Haven, Connecticut= 3'-6" down

## Foundation Calculations:

### Total Load:

1. Calculate the **weight of the building loads** on the foundation (given = BL)

### Structure Area:

1. Calculate the **number of columns** distributing the load: (count columns = CO)

2. Calculate the **area of each column** (CA = length x width):

3. Multiply the number of columns x area of each column to find **total load distribution area** (TLDA = CA x CO)

4. Divide the Total building Loads by the load distribution area to find the **amount of load needed to spread** per square foot (BL/ TLDA = LS)

### Soil Bearing Pressure:

1. Look up the Allowable soil bearing pressure **from the chart** given (ASBP)

2. If the **LS > ASBP**, then spread footings are needed.

## Total Bearing Area/ Footing size:

1. Calculate Total Loads (BL) / Allowable soil bearing pressure to find **bearing area needed** ( $BA = BL / ASBP$ )
2. Divide the Bearing Area needed by the # of Columns to get the **size needed per footing/ pile/ caisson**. ( $BA / CO = SPF$ )
3. **Calculate the dimensions** of the footing/ pile/ caisson (Cylinder:  $SPF = 3.14 R^2$ ) (Rectangle = Square root of SPF)